

SUMMER- 18 EXAMINATION

Subject Name: ESTIMATING AND COSTING Model Answer

Important Instructions to examiners:

1) The answers should be examined by key words and not as word-to-word as given in the model answer scheme.

Subject Code: 17501

- 2) The model answer and the answer written by candidate may vary but the examiner may try to assess the understanding level of the candidate.
- 3) The language errors such as grammatical, spelling errors should not be given more Importance (Not applicable for subject English and Communication Skills.
- 4) While assessing figures, examiner may give credit for principal components indicated in the figure. The figures drawn by candidate and model answer may vary. The examiner may give credit for any equivalent figure drawn.
- 5) Credits may be given step wise for numerical problems. In some cases, the assumed constant values may vary and there may be some difference in the candidate's answers and model answer.
- 6) In case of some questions credit may be given by judgement on part of examiner of relevant answer based on candidate's understanding.
- 7) For programming language papers, credit may be given to any other program based on equivalent concept.

Q.	Sub	Answers	Marking
No.	Q. N.		Scheme
Q.1	a)	Attempt any THREE of the following:	
	(i)	State the meaning of the term estimating and costing.	
	Ans	It is the process of calculating probable quantities of various items and expected	04 Marks
		expenditure to be incurred for particular project or work.	
Q.1	a)(ii)	Enlist the types of estimate. Mention the situation when revised estimate is prepared.	
	Ans	Types of estimate:	
		1. Approximate estimate.	
		a. Plinth area method.	
		b. Cubic content method.	
		c. Service unit method.	01 M
		d. Approximate quantity method.	
		e. Typical bay method.	
		2. Detail estimate.	
		a. Original or new estimate.	
		b. Revised estimate.	
		c. Supplementary estimate.	01 M
		d. Annual repairs and maintenance estimate.	
		Revised estimate is prepared in following situations.	
		1. When original sanction amount exceeds or likely to exceed by more than 5% from	
		the rates in original estimate.	02 Marks
		2. When the expenditure of work exceeds or likely to exceed by more than 10% of	(1/2 per
		administrative approval.	situation)
		3. When there is material deviation from original proposed.	
		4. When sanctioned estimate is more than requirement	
Q.1	a)(iii)	State mode of measurements for the following items of work.	
		I) Barbed wire fencing2) Skirting3) Dado4) Purlins	



	Ans	Modes of me	asurement:							04 Marks
		1. Barbe	d wire fencing: Running m	eter.						(01 for
		2. Skirtir	 Skirting: Running meter (depth 15 cm or mentioned) OR Square meter. Dado: Square meter. 							
		3. Dado:	Square meter.				, .			
		4. Purlin	s: Running meter OR Cubic	: meter.						
Q.1	a)(iv)	Prepare appr	oximate estimate of a build	ding usi	ng foll	owing	g data.			
		1) Proposed a	area of the building 150 sq.	.m.						
		2) Similar typ	es of building is recently co	onstruct	ed in	nearb	y locality h	aving buil	t-up area	
		110 sq.m. and	d the total cost of construc	tion is F	Rs.12	akhs.				
	Ans	Built up area	rate = 12,00,000 / 110 = 1	0,909.1	per S	q. M.				02 M
		Approximate	cost of proposed building	= 10,90	9.1 x 1	.50 = F	Rs. 16,36,3	65/-		02 M
Q.1	b)	Attempt any	ONE of the following:						_	
	(i)	Draw the star	ndard formats of measurement sheet, abstract sheet; and face sheet. State use							
		of face sheet.								
	Ans	Measuremen	t sheet:		<u> </u>		D 111			
		Item	Description of item	No.	Len	igth	Breadth	Height	Quantity	01 14
		NO.	· · · · · · · · · · · · · · · · · · ·			L	В	D/H		
		Abstract shoe	×+•							
								Unit of		
		No	Description of item	Quar	ntity	Unit	Rate	rate	Amount	01 M
		110.						Tate		01.111
		Division: Sanction estin Fund head: Major head: Minor head: Service head: Departmenta Estimate will occur in N Administrativ Technical san Estimate prep And checked	mate No.:		ingine ted ed	er, P.\	W.D., the p 	orobable e 	xpenses that	01 M 01 M
		Call of author	rity:							
			G	eneral a	bstra	ct				
		Sr. No.	Partie	culars				Amou	nt	
		1	Estimated cost (as per at	ostract)			Rs	•		
		2	Water supply and Sanita	ry charg	ges @	9	% Rs	•	_	
		3	Electrification charges @)%			Rs	•		
		4	Contingencies @ 3 to 5%	, b			Rs	•	_	
		5	Work charged establishn	nent @	1 to 2	%	Rs	•		
			Total estimated cost				Rs	•		



		Use: It	is used t	o mention details of project for which estim	nate is prepared and attached at	01 M				
		cover	page of d	letail estimate.						
Q.1	b)	State t	he rules	for deductions as per IS 1200 for						
	, (ii)	1) Mas	1) Masonry work in superstructure 2) Plastering							
	Ans	1) Mas) Masonry work in superstructure:							
		, No de	, duction s	hall be made for						
			• Opening up to 0.1 Sq.m. in area.							
			• Ends	of beams, lintels, post up to 0.05 Sq.m.						
			• Wall	plates, bed plates, bearing of slab etc. when	n thickness does not exceed 10					
			cm.							
		2) Plas	stering:							
		i. No d	. No deductions for opening up to 0.5 Sq. m. each and no addition made for jambs, soffits							
		and sil	and sills of those openings.							
		ii. For	i. For opening more than 0.5 Sq.m. but not exceeding 3 Sq.m.							
		•	50% of 0	opening area shall be deducted from each fa	ace if breadth of sill is same on					
			each f	ace <u>OR</u>		02 M				
		•	Full ope	ning area shall be deducted from the face o	n which breadth of sill is less.					
		iii. For	opening	more than 3 Sq.m. full deduction shall be m	ade from each face but separate	01 14				
		additio	ons shall	be made for jambs, soffits and sills.						
Q.2	,	Attem	pt any TV	VO of the following:	c					
	a)	Descri	be the pr	ocedure for preparing approximate estimat	e of a water supply project.					
	Ans	1)	I) Basis of preparing approximate estimate of Water supply project may be population							
			to be served by project.							
		ner ca	nita can k	be found out, and then approximate cost of	new project can be calculated					
			Otherwi	ise water supply project is divided in to follo	wing units and approximate cost	02 M				
		,	ofe	ach unit is found out and then total approxi	imate cost of project is	02 111				
			calc	ulated.						
			Sr. No.	Unit	Service unit					
			1	Intake or head work	Per MLD OR Per m ³ capacity					
			2	Pumping machinery	Per H.P. <u>OR</u> KW					
			3	Rising main	Per running mater					
				Treatment unit like aerator, flash mixer,						
			4	clarifloculator, rapid sand filter,	Per MLD <u>OR</u> Per m ³ capacity					
				disinfection.						
			5	Ground storage reservoir and ESR	Per liter capacity	04 M				
			6	Distribution system	Per running meter					
			7	Staff quarter	Per Sq. m.					
			8	Land acquisition	Per Acre					
Q.2	b)	Prepa	re approx	imate estimate of a bridge having 4 spans o	f 50 m each using following data.					
		(i) Cos	t of existi	ng bridge Rs.l.5 cr. (ii) Existing bridge ha	aving 3.3 spans of 60 m each.					
	Ans	Length	n ot existi	ng bridge = No. of spans x length of each sp	an	0.00				
				= 3.3 X 6U		02 M				
		Data -	or motor	= 198 M.						
		гасе р	ermeter	- 1 50 00 000 / 109						
				- 1,50,00,000 / 150 = 75757 58		02 M				
1	1	1		, , , , , , , , , , , , , , , , , , , ,		02 101				



		Length of new	bridge :	= 4 x 50 = 3	200 m						02 M
		Approximate of	cost of n	ew bridge	$= 200 \times 75$	5757.58					02 M
0.2		The formation		rood of ct	= KS. 1515	01510. ot is 470.00	0 m Thore		faca chall k	o folling	
Q.Z	()	gradient line o	$f_1 + c_6$	10au al Si O formatic	arting poir on width o	f road is 12) m Side d	ono 1.1	idce slidil k Din omban	kment and	
		1.1 5 in cuttin	σ σ			110au 15 12	III. Slue Sl	ope 1.2			
		Assume there	5·· is no cra	nss slone t	o the grou	nd					
		Chain	age in M		30	60	90	1	20 1	50	
		RL of	Glinn	n <u>165</u> 0	0 467.2	0 468 1	0 468.20	1 16	9 70 / 160	3 00	
				of earthw	ork For rc		lean Secti	nal Ar	a Methor	1	
	Ans	Fall in gradien	+ 1·60	orcartitiv							
		Fall in formati	on level	for each 3	0 m = 30 /	′60 = 0 5 n	า				
		Chainage in	M	0	30	60	90	1	20	150	
			n m	465.00	467.20	468 10	468.20	469	$\frac{10}{10}$ $\frac{10}{10}$	59.00	
		Formation le	Formation level 470.0 469.5 469.0 468.5 468.0 467.5 02								02 M
		Height o	Height of								
		embankme	ent	5.0	2.3	0.9	0.3	-			
		Depth of cut	Depth of cutting 1.7 1.5								
		· ·	0								
						0.3					
						00		30 - X —		20	
		X / 0.3	= (30 - 2	X) / 1.7		90—X—			1	20	
		1.7X =	9 – 0.3X								
		2X = 9									01 M
		X = 4.5									
		No bar	nking an	d no cuttir	ng at						
		section	י = 90 + י	4.5 = 94.5	m				1.7		
			Daula	A	rea	Mea	n area		Vol	ume	
		Chainage	Deptn	Banking	Cutting	Doubling	Cutting	L	Doulting	Cutting	
			a	S=2	S=1.5	Banking	Cutting		Banking	Cutting	
		0	5.0	110							
		30	2.3	38.18		74.09		30	2222.7		04 M
		60	0.9	12.42		25.3		30	759		
		90	0.3	3.78		8.1		30	243		
		94.5	0	0		1.89		4.5	8.505		
		120	-1.7		24.735		-12.3675	25.5		315.37	
		150	-1.5		21.375		-23.055	30		691.65	
								Total	3233.205	1007.02	
		Earth v	vork in k	anking = 3	3233.205 (Cu.m.					
		Earth v	vork in c	utting = 1	007.02 Cu.	.m.					01 M
Q.3		Attempt any F	OUR of	the follow	ing:	10					
	a)	How will you p	ow will you prepare estimate for irrigation canal?								
	Ans	Estimate for in	rigation	canal.	а		4.0.0.1	.		al is seen	
		ine unit to be	adopted	a for findir	ig out the	approxima	ite estimat	e or irr	igation car	ai, is one	



		of the following							
		a) Area of land under comm	nand of canal		01 14				
		b) Per Km length.							
		In the first case, the area under the	e command of irrigation canal is	worked out in hectares.					
		Knowing the cost of similar project	, suitable amount per hectare is	decided. The					
		approximate estimate is calculated	l by multiplying the area under th	ne command to per					
		hectare cost of canal.			02.14				
		In second case cost per Km length	is calculated from the similar uni	ts, constructed	02 101				
		previously.							
		The approximate cost of proposed	canal is calculated by multiplying	g the length of proposed					
		canal to cost per km length of cana	3). 						
		An amount of contingencies, norn	hally 10% is added to the cost of	project. For overneads,	01 M				
		10%cost of approximate estimate	is included in the estimate.	annrovimato actimatod	01 101				
		At last cost for land acquisition, no	initially 12% is added to get total	approximate estimated					
0.2	b)	Describe D S R. State its uses							
Q.3	D) Ans	$D \subseteq B$: - A list of rates of various its	ams is prepared to facilitate prep	aration of estimate by					
	Alls	government hodies like Public Wo	rks Department As the rates var	from place to place	01 M				
		Maharashtra Government nublish	es list of rates as per districts. The	ese rates are in the form					
		of printed booklet and called as Di	strict Schedule of Rates (DSR).						
		This booklet is revised every ve	ar because of changes in cost of	abor. material everv					
		vear.							
		It includes Completed rates, per	unit cost of item of work and La	bor rates.					
		Per unit cost of item includes cost	of material, cost of labor, transpo	ortation charges, storage	02 M				
		of material, charges for tools mach	ineries and plants etc.		02 101				
		Labor rates include charges to b	e paid to head mason, mazdoor,	coolie etc. depending on					
		the category of labor.							
		It also includes initial lead and li	ft and separate charges are appl	cable for more lead and					
		lift. Similarly the rates are applicab	le to ground floor only and they	are increased for each					
		upper floor.							
		Uses of DSR: a) The work carried o	ut by the department is estimate	d according to DSR.	01 M				
		b) Rates shown in DS	R Reep check on rates quoted by	the contractor.					
0.2	c)	Cive the bire charges for following	machiner //equipment	tender may be rejected.					
Q.5	C)	(i) Concrete mixer (ii) Dumpe	r (iii) Vibrator						
	Δns	(i) concrete mixer (ii) bumpe							
	Alls	Machinery/Equipment	Hire charges						
		Wideliner y/Equipment							
		1) Concrete Mixer	Rs. 600 to Rs. 800 per day						
		2) Dumper	Rs. 1000 to Rs. 1200 per day		01 M for				
		3) Vibrator	Rs. 400 to Rs. 500 per day		each				
		4) JCB	Rs. 800 to Rs. 1000 per hour						
		Note:- Hire charges may yory from	nlace to place						
		Note:- Hire charges may vary from place to place.							



Q.3	d)	State the desired accuracy in taking measurement of work as per IS 1200.	
	Ans	To achieve the desired accuracy in measurements, following points must be observed.	
		1) Dimensions shall be measured to the nearest 0.01m except	
		a) Thickness of slab measured nearest to 0.005m	02 M
		b) Wood work is to be measured nearest to 0.002m	02 101
		c) Reinforcement , to the nearest 0.005m	
		d) Thickness of roadwork less than 200mm is measured nearest to 0.005m.	
		The tolerances in measurements are	
		a) For volumes 0.01 cu.m	02 M
		b) For areas0.01 sq.m	02 101
		c) For lengths 0.01 rmt	
		d) For weights0.001 ton or 1kg.	
		Fraction less than one half is neglected.	
		Fraction equal to one half or more than one half is considered	
Q.3	e)	Explain the long wall and short wall method for taking out quantities.	
	Ans	In this method longer walls in building in one direction are consider as long wall and it is	
		measured out to out. Walls in perpendicular direction of long walls, are consider as short	02 M
		walls and measured in to in for a particular layer of work. This is most practical method as it	
		can be used under all circumstances. Following steps are involved in this method.	
		1) Foundation plan showing centre line with all dimensions. Centre to centre length is	
		calculated by adding half width of each cross wall to inner dimensions of a room	
		2) Group the walls as long walls and short walls. Measure the length of long wall for an item	
		2 Group the wais as long wais and short wais. Measure the length of long wait for an item using equation length of long wall – c/c length of long wall + width of item at that layer	
		2) Moasure the length of short wall for an item using equation longth of short wall $= c/c$	
		Sy measure the length of short wall for an item using equation, length of short wall – c/c	02 M
		4) Multiply number of walls longth broadth and donth to get the guantity of item	
		This method is simple, quick and accurate. Method is also known as DM/D method. At eveny	
		This method is simple, quick and accurate. Method is also known as PWD method. At every	
		of chort wall increases	
0.2	f)	State nurnese of supplementary estimate. Give one example	
Q.5	1) Ans	1) During the execution of project, cortain new items or additional works crop up to	
	Alls	supplement the original project. Under such circumstances, it becomes necessary to	
		supplement the original project. Onder such circumstances, it becomes necessary to	
		2) Supplementary estimate is prepared for sovering the estimate of sub work of a	03 M
		2) Supplementary estimate is prepared for covering the estimate of sub-work of a	
		project, which is considered necessary for full development of project.	
		3) Sometimes changes due to material deviation of a structural nature from the original	
		approved design are necessary when the work is in progress. Then for all such items	
		supplementary estimate is prepared.	
		Example: If in a bed room of Bungalow of executive engineer, attached toilet is not	
		provided in original project and then it is decided to construct attached toilet, then	01 M
		supplementary estimate is necessary.	



Q.4	a)	Work (i)Exca	out quantities of the following any avation for foundation (i	THRE i) Bric	E items k work	of wor	k from er struc	Figure No.1. ture in c.m.	(1 :6)			
	Ans	(iii) Inf C/C di C/C di C/C di C/C di	ternal plaster in c.m. (1 :4) (i stance AB = 4.1 + 0.3 = 4.4 m. stance CD = 7.5 + 0.6 = 8.1 m stance AE = 3.2 + 0.3 = 3.5 m stance EC = 4.2 + 0.3 = 4.5 m	v) R.C	.C. slab	(1:2:4))3.5m	<u>É</u> 4.5m	C 2.8m 5.3m			
			Measurement Sheet	rement Sheet					CENTER LINE PLAN			
		S.N.	Description	No	L	В	D/H	Qty	Total Qty			
		1)	Excavation for foundation AB = 4.4+1.0=5.4 CD= 8.1+1.0=9.1 AE= 3.5-1.0=2.5 EC =4.5-1.0=3.5	1 2 2 3	5.4 9.1 2.5 3.5	1.0 1.0 1.0 1.0	1.15 1.15 1.15 1.15	6.21 m ³ 20.93m ³ 5.75 m ³ 12.075m ³	44.965m ³			
			<u>OR</u> by center line method Total center line length = 41.1 m Effective center line length = $41.1 - 4 \times 1.0/2 = 39.1$ m.	1	39.1	1.0	1.15	44.965m ³	44.965m ³			
		2)	Brick work in superstructure in CM (1:6) AB = 4.4+0.3=4.7 CD= 8.1+0.3=8.4 AE= 3.5-0.3=3.2 EC =4.5-0.3=4.2 Deductions Door D Windows W1 Windows W	1 2 3 3 5 2	4.7 8.4 3.2 4.2 1.0 1.2 1.8	0.3 0.3 0.3 0.3 0.3 0.3 0.3 0.3	3.0 3.0 3.0 3.0 2.1 1.2 1.2	4.23 m ³ 15.12 m ³ 5.76 m ³ 11.34 m ³ -1.89 m ³ -2.16 m ³ -1.296m ³	31.104m ³	Any Three		
			<u>OR</u> by center line method Total center line length = 41.1 m Effective center line length = $41.1 - 4 \times 0.3/2 = 40.5$ m. Deductions	1	40.5	0.3	3.0	36.45 m ³ -5.346m ³	31.104m ³	each		
		3)	Internal plaster in CM (1:4) Ceiling Bed room Kitchen Living Internal faces of wall	1 1 1	3.2 4.2 4.2		4.1 2.5 5.0	13.12 m ² 10.5 m ² 21.0 m ²				



					-		-			· .	
			Bed room	2	3.2		3.0	19.2 m ²			
				2	4.1		3.0	24.6 m ²			
			Kitchen	2	4.2		3.0	25.2 m ²			
				2	2.5		3.0	15.0 m ²			
			Living	2	4.2		3.0	25.2 m ²			
				2	5.0		3.0	30.0 m ²			
			Deductions								
			Doors	2.5	1.0		2.1	-5.25 m ²			
			Windows W1	2.5	1.2		1.2	-3.60 m ²			
			Windows W	1	1.8		1.2	-2.16 m ²	172.81m ²		
		4)	R.C.C. Slab (1:2:4)	1	8.4	4.8	0.15	6.048 m ³			
				1	4.7	3.5	0.15	2.468 m ³	8.516 m ³		
Q.4	b)	Attem	pt any ONE of the following:						<u> </u>		
	(i)	Find o	ut the quantities of cement, sand a	and ag	gregate	e for R.	C.C. 1:2	:4 work of 2	5 cu.m		
	.,	quant	ity.		0 0						
	Ans	Quant	ities of cement, sand and aggregat	e.							
		Wet v	olume of concrete given is 25 cu.m								
		Add 5	2% more for voids and wastage to	get dr	y volum	າe				01 M	
		Dry vo	olume = 25 + 25 (52/100) = 38 cu.m							01 M	
		Concr	ete is in proportion 1:2:4, hence								
		Quant	Quantity of cement = [dry volume / (1+2+4)] x part of cement								
		= (38/7) x 1 = 5.43cu.m 02									
		Number of bags= 5.43 / 0.035 = 155.1 bags = 155 bags OR 156 bags 01									
		Quant	ity of sand = (38 / 7) x 2 = 10.86 cu	.m						01 M	
		Quant	ity of aggregate = (38 / 7) x 4 = 21.	71 cu.	m					01 M	
		Note:	Someone may take volume of 1 b	ag of (cement	as 0.03	34 m ³ .				



Q.4	b)(ii)	A R.C mm c mm c The o	.C. simply suppo liameters main liameter are pro verall beam len	orted beam of size 295 bars are placed in one ovided at top. 8 mm di ogth is 6 m. Calculate th	5 mm x row ar amete he tota	645 mm is nd two ben rs stirrups a l quantities	reinforced t up. Two are provide of the ste	d with 4 Anchor k ed at 14(Nos. of 20 Dars of 12 Dmm c/c. red. Show		
	Ans	bar b Data Lengt Main Anch Stirru	ending schedule given: Beam siz h of beam = 6m bars: 4 no of 20 or bars at top: ps : 8 mm dia. p	es. e 295mm x 645 mm)) mm diameter out of two anchor bars of 12 provided at 140 mm c/	which t mm dia ′c	two bent up).				
		Assu Bars 180 ⁰	ime cover to the are bent up at 4 Hook is provid a) Calculat L = 6000	e reinforcement 25 mr 45 ⁰ ed on both sides e length of one straigh) - 2 x 25 + 2 x 9 x 20 = –	n from t bar w 6310 n	all sides. vith 180 ⁰ ho nm	ook on bot	h sides.		01 M	
			b) Length o L= 6000 -	- f bent up bars with 18 2 x 25 + 2 x 9 x 20 + 2	0 ^{.31} m 0 ⁰ hoo 2 x 0.42	k on both s x (645 - 50	ides.) = 6809.8 = 6.81m	0mm		01 M	
	c) Length of anchor bar at top with 180° hook on both sides. L = 6000 -2 x 25 + 2 x 9 x 12 = 6166 mm = 6.17 m										
	d) length of 2 legged stirrup L = 2 x (295 – 2x25) + 2 x (645 – 2x25) + 24 x 8 = 1872mm = 1.87 m No of stirrups = [(6000 - 2 x 25)/140] + 1 = 43.5 = 44 nos Bar bending schedule:										
		S.N	Particulars	Shape of bar	No of bars	Dia.of Bar(mm)	Total length (m)	Wt of Bar in kg/m	Total wt In kg		
		1)	Top anchor bars with hook on both sides,		2	12mm	12.34m	0.89	10.98 kg		
		2)	Two straight bars at bottom with hook on both sides		2	20mm	12.62m	2.47	31.17kg	02 M	
		3)	Two bent up bars at 450	$\overline{}$	2	20mm	13.62m	2.47	33.64kg.		
		4)	Two legged stirrups		44	8 mm	82.28m	0.39	32.09kg.		
								Total	107.88kg.		



Q.5		Attempt any TWO of the f	ollowing:							
	a)	Prepare rate analysis for p	lain cement cor	ncrete of gra	de M15 (1:2:	4).				
	Ans	As	sume Wet volu	me of Concre	ete = 10 Cu. r	n.				
		(A) Calculation of material	s :							
		Therefore, D	Dry volume of co	oncrete = 52	% more of w	et concrete		01 M		
			= 10 + ((52/10	00) x 10) = 15	5.20 cu. m.					
			PCC Grade M15	having prop	ortion 1:2:4					
		(1) Volume of ce	ment = (dry volu	ume/sum of	proportion) >	<pre>c part of ceme</pre>	ent			
			= (15.20/(1+2-	+4)) x 1 = 2.1	714 cu. m.			01 M		
		Therefore no. of	f cement bags =	volume of c	ement / vol.	of cem. Per ba	эg			
			= 2.1714 / 0.03	5 = 62.04 say	62.50 bags.					
		Note: Someone may take	volume of 1 ba	g of cement	as 0.034 m [°] .			04.14		
	(2) Volume of sand = (dry volume/ sum of proportion) x part of sand. = $(15.20/(1+2+4)) \times 2 = 4.343 \text{ cu. m.}$									
		(3) Volume of coarse aggregate. = (dry volume/ sum of proportion) x part of agg.								
		(D)Table for rate applysis f	= (15.20/(1+2))	(+4)) x 4 = 8.	586 cu. m.					
		(B) Table for rate analysis i	Or 10 Cu. m.	Data nor	linit of	Amount				
		Particulars	Quantity	Kate per	mosurts	AIIIOUIIL (Pc.)	l.			
				unit	mesurts.	(KS.)				
		(A) Material :		5 000		20027-00	l.			
		Cement	62.50 bags	Rs. 330	bag	20625.00	l.			
		Sand 4.343 cu. m. Rs. 800 Cu. m. 3474.40								
		Coarse Aggregate	8.686 cu. m.	Rs. 800	Cu. m.	6948.80				
		(B) Labour :								
		Head mason (Mistri)	1/2 no.	Rs. 500	day	250.00				
		Mason	2.5 no.	Rs. 400	day	1000.00				
		Male Mazdoor	12 no.	Rs. 300	day	3600.00				
		Female Mazdoor	18 no.	Rs. 250	day	4500.00				
		Bhisti (including curing)	6 no.	Rs. 250	day	1500.00		04 Marks		
		Forms etc. (as per req.)	Lump sum	1800 L.S.		1800.00		for Table		
		Sundries T and P etc.	Lump sum	300 L.S.		300.00	l.	and		
			Tota	l of Material	and Labour	43998.20		values.		
			ŀ	Add 1.5 % wa	ater charges	659.97	l.			
			Ado	l 10 % contra	actors profit	4399.82				
			Grand To	otal (Rate pe	r 10 Cu. m.)	49057.99	l.			
				Rat	e per Cu. m.	4905.79	l.			
					Say	Rs.4906.00				
		(Note: Assumption can be	e made by unde	rstanding of	student. Rat	te may vary fi	om place			
		to place.)								
Q.5	b)	Workout the quantity of the	he following her	ms. For septi	c tank having	g internal size	1.6 m x 3.8			
		m having height 1.5 m. Th	e top of slab of	septic tank is	s 20 cm. abov	ve G.L.				
		(i) Earthwork in Excavatior	ר (ii) ו	P.C.C. (1:3:6)	15 cm thick.					
		(iii) B.B. Masonry in C.M. (1:6) of 200 mm	thick.						
	(iv) R.C.C. slab (1:2:4) on septic tank 12 cm thick.									
	Ans	First of all , draw the plan	and sectional el	evation of Se	eptic tank fro	m the given d	ata			
	1							1		







			= (4.20/(1+	4)) x 1 = 0.8	4 cu. m.			01 M
		Therefore no. c	of cement bags =	volume of o	cement / vol.	of cement ba	ıg	
			= 0.84 /	0.035 = 24 k	bags.			
		(2) Volume of sand = (dry	volume/ sum of	proportion)	x part of san	d.		
			= (4.20/(1+	4)) x 4 = 3.3	6 cu. m.			01 M
		(3) Volume of Stone = 25 9	% more of volun	ne of mason	ry.			
			= ((25/100) x	10) + 10 = 12	2.50 cu. m.			01 M
		(B)Table for rate analysis f	or 10 Cu m					
		Particulars	Quantity	Rate ner	Unit of	Amount		
			Quantity	unit	mesurts.	(Rs.)		
		(A) Material :		Gint	mesures	(1.01)		
		Cement	24 bags	Rs. 330	bag	7920.00		
		Sand	3.36 cu. m.	Rs. 800	Cu. m.	2688.00		
		Stone including						
		through bond stone	12.50 cu. m.	Rs. 1100	Cu. m.	13750.00		
		and wastage						
		(B) Labour :						
		Head mason (Mistri)	1/2 no.	Rs. 500	day	250.00		
		Mason	12 no.	Rs. 400	day	4800.00		04 M for
		Male Mazdoor	10 no.	Rs. 300	day	3000.00		U4 IVI IUI Tablo
		Female Mazdoor	10 no.	Rs. 250	day	2500.00		and
		Bhisti	1.5 no.	Rs. 250	day	375.00		values
		Scaffolding	Lump sum	400 L.S.		400.00		Values
		Sundries T and P etc.	Lump sum	300 L.S.		300.00		
			Tota	l of Material	and Labour	35983.00		
			ŀ	Add 1.5 % wa	ater charges	539.75		
			Ado	l 10 % contra	actors profit	3598.30		
			Grand To	otal (Rate pe	er 10 Cu. m.)	40121.05		
				Rat	e per Cu. m.	4012.11		
					Say	Rs.4012.00	j	
		(Note : Assumption can be	e made by unde	rstanding of	student. Rate	e may vary fro	om place to	
0.6		Attempt any FOLIR of the	following					
Q.0	a)	Defme 'Task work'. Enlist a	any four factors	affecting tag	sk work.			
	Ans	Task Work: The capacity o	f doing work by	an artisan o	r skilled labo	ur in the form	n of quantity	
		of work per day is known	as the Task-Wor	k or Out-tur	n of the labo	ur.		02 M
		 Factors affecting ta 	ask work of the	abour:				
		Task work of a skilled labo	ur depends upo	n the follow	ing factors:			
		(1) Nature, Size, Heigh	nt, situation, loc	ation, clima	tic condition,	techniques a	adopted and	
		wages paid etc.				·	-	Any four
		(2) Availability of skille	d labour.					(1/2 M
		(3) A well-organized w	ork.					each)
		(4) Job satisfaction or	working condition	ons.				
		(5) Allotment of piece	of work.					



Q.6	b)	State the names of software that are used for preparation of detailed estimate of building	
		work.	
	Ans	Following are the names of software that are used for preparation of detailed estimate of	
		building work.	
		1. Sage Estimating	
		2. CoConstruct	
		3. Buildertrend	
		4. Clear Estimates	
		5. MEP Estimating	
		6. Corecon	
		7. Sigma Ai	ny eight
		8. Esticom	(1/2 M
		9. Estimator	each)
		10. STACK	-
		11. Build-Quant	
		12. Build-Master	
		13. Civil estimator	
		14. Turbo Bid	
		15. Intelli Bid	
		16. Pro Est	
		17. B2W (BID2Win)	
		(Note : The names of software may vary with student knowledge. Therefore marks should	
		be given with respect to that.)	
Q.6	c)	What are the different methods used for calculation of earthwork quantities for a road and	
		canal? Explain any one.	
	Ans	The different methods used for calculation of earthwork quantities for road and canal are	
		as follows:	
		1. Mid-sectional area method	
		2. Mean Sectional area method02	02 M
		3. Prismoidal formula method	
		4. Trapezoildal formula method	
		(1) Mid-sectional are method: In this method, the mid-section area is calculated by dividing	
		the trapezoidal cross-section of Road/Canal into rectangle and two triangles and then this	
		mid-section area is multiplied by the length of the section to get quantity of earthwork as	
		given below:	
		Area of mid section = Area of rectangular	
		portion + area of two	
		$= Bd_{+} + \frac{1}{4sd_{+}^{2} + \frac{1}{4sd_{+}^{2}}} = Bd_{+} + \frac{1}{sd_{+}^{2}}$	
		Ountity of earthwork = $(\text{Rd} + \text{id} 2) \times 1$ Fig 7-4	
		General $Q = (Pd + sd^2) \times L$ where d stands for mean height or denth	
		$Q = (Bd + so -) \times L$, where d stands for mean neight of depth.	
		Stations Depth Mean Area of Area of Total Length Quantity 02	2 M
		or or Depth central sides Sectional between $(Bd + sd^2) \times L$ (F	For any
		Chain-Height or portion Sd ² Area stations Height Bd Bd+sd ² I Embank Cutting	ne
		"d" bu	nethod)
		the second se	



(2) Mean Sectional area method : In this method, cross sectional area at two ends of section is calculated and then mean of these two is multiplied by length of section to get volume of earthwork.

The m	can section	al area A =	$\frac{A_1+A_2}{2}, Q$	uantity Q =	$\frac{A_1 + A_2}{2} \times L$	ength.	Laborer	
Stations	Height	Area of	May be call	Total	Mean Sectional	Length	Quantity (Bd+sd ²) × L	
Stations	Height	Area of	Area of	Total	Mean	Length	(Bd+so	1 ²) × L

(3) Prismoidal Formula Method: In this method, the cross sectional area at the two ends of a portion of embankment is calculated and then mid sectional area is also calculated. The quantity or volume can be calculated by following formula:

Quantity or volume = $(L/6) \times (A_1 + A_2 + A_m)$

Where A_1 and A_2 are the cross sectional areas at the two ends of a portion of embankment of road or canal of length L, A_m is the mid-sectional area.

Let d_1 and d_2 be the heights of banks at the two ends, and d_m be the mean height at the mid-section, B be the formation width and S:1 be the side slope. Then



Q.6d)Define' rate analysis and state the factors affecting rate analysis.AnsRate analysis : The determination of rate per unit of a particular item of work, from the cost
of quantities of materials, the cost of labourers and other miscellaneous petty expenses
require for its completion is known as the rate analysis.02

01 M



*Factors affecting the rate analysis :- The factors which affect the rate analysis of an item can be broadly divided into	
following :	
(1) Major Factors and (2) Minor Factors	
(1) Major factors : The are mainly two factors on which the rate of an item depends,	
(I) Materials and (II) Labour.	
(1) Materials :- The quantities of various materials required for the construction of an item can be	01 M
easily worked out by knowing the specification of that item.	UT IVI
(ii) Labour :-	
The labour force will be necessary to arrange the materials in a proper way so that the item	
can be completed.	
(2) Minor Factors :-	
(i) Special equipment: - If the execution of an item requires the use of some special	
equipment ort plant, the cost of using such special equipment on the rental basis should be	
included in the rate analysis of that item.	
(ii) <u>Place of work</u> :- The site of work will also have some effect on the rate of an item under	
certain conditions. If it is too far, more amount will have to be spent on carting. This will	
are to be modified	any four
(iii) Nature of work :- If the work consists if large quantities of the items, the rates may be	1/2 M
less and vice versa.	each
(iv) Conditions of contract :- If the condition of contract are very stiff, the rates of various	
items will be high and vice versa.	
(v) <u>Profit of the contractor</u> :- The usual percentage of the profit of the contractor is TEN. But	
if it is more or less, the rate of the item will be correspondingly affected.	
(vi) <u>Specifications</u> :- If the specifications of work provide for rigid type tolerances and	
superior quality turn out, the rates will be on the higher side.	
(VII) Site conditions :- If the site conditions are such that difficulties will be experienced	
during execution of work, such as foundations involving water troubles, thue rates will be	
construction activities the contractor may quote slightly lower rates	
(viji) Miscellaneous :- The other remaining miscellaneous factors affecting rates of items	
include time of completion of the project, climatic conditions, reputation of the contracting	
firm, discipline of the organization, etc.	



e) Ans	Calcul in Figu (i) Exc	ate the quantities o ure No.2.	f follov	ving items o	of work for a	a circular	community wel	as shown							
Ans	in Figi (i) Exc	ure No.2.						Calculate the quantities of following items of work for a circular community well as shown							
Ans	(i) Exc		Figure No.2.												
Ans		cavation in Soft Murum. (ii) R.C.C. Ring Beam quantity of concrete.													
	From	m the Figure no. 2													
	Qty.	of Excavation and co	oncrete	is calculate	ed in Table k	elow:									
	Sr.	Item of work	Nos.	Length	width	depth									
	No.			OR	OR Area		Quantity								
	(i)	Excavation in soft murum													
	1	i) up to 1.5 m depth	1	((π/4) x 5.	20 ²) sq. m.	1.5 m	31.86 cu. m.		02 M						
	2	ii) 1.5m to 3.0 m depth	1	((π/4) x 5.	20 ²) sq. m.	1.5 m	31.86 cu. m.		(01 M for lift wise cal. And						
	3	iii) 3.0m to 4.5 m depth	1	((π/4) x 5.	20 ²) sq. m.	1.5 m	31.86 cu		01 M for its total)						
	4	iv) 4.5m to 6.0 m depth	1	((π/4) x 5.	20 ²) sq. m.	1.5 m	31.86 cu. m.								
	5	iv) 6.0m to 7.5 m depth	1	((π/4) x 5.	20 ²) sq. m.	1.5 m	31.86 cu. m.								
	6	iv) 7.5m to 8.5 m depth	1	((π/4) x 5.	20 ²) sq. m.	1.0 m	21.24 cu. m.								
		Total excavation of soft rock 180.54 cu. m.													
	(ii)	R.C.C. Ring beam (RCC M20)													
		The Ring Beam has size of 0.3 m x 0.3m. The inner diameter of well is 4.60 m and Outer diameter is 5.20 m.													
	1	RCC quantity in Ring Beam	1	(π/4) x 4.60 ²)	(5.20 ² – sq. m.	0.3 m.	1.39 cu. m.		02 M						
	Total Quantity of RCC in Ring Beam 1.39 cu. n						1.39 cu. m.								
		Sr. No. (i) 1 2 3 4 5 6 (ii) 1	Sr. No.Item of work(i)Excavation in soft1i) up to 1.5 m depth2ii) 1.5m to 3.0 m depth3iii) 3.0m to 4.5 m depth4iv) 4.5m to 6.0 m depth5iv) 6.0m to 7.5 m depth6iv) 7.5m to 8.5 m depth(ii)R.C.C. Ring beam (not show the sho	Sr. No.Item of workNos.(i)Excavation in soft murun1i) up to 1.5 m depth12ii) 1.5m to 3.0 m depth13iii) 3.0m to 4.5 m depth14iv) 4.5m to 6.0 m depth15iv) 6.0m to 7.5 m depth16iv) 7.5m to 8.5 m depth16iv) 7.5m to 8.5 m depth17II8II9II10II11II12II13II14II15II16II17II18II19II10II11II12II13II14II15II16II17II18II19II10II11II12II13II14II15II16II17II18II19II10II11II12II14I <tdi< <="" td=""><td>Sr. No.Item of workNos.Length OR(i)Excavation in soft murum1i) up to 1.5 m depth1$((\pi/4) \times 5.1)^2$2ii) 1.5m to 3.0 m depth1$((\pi/4) \times 5.1)^2$3iii) 3.0m to 4.5 m depth1$((\pi/4) \times 5.1)^2$4iv) 4.5m to 6.0 m depth1$((\pi/4) \times 5.1)^2$5iv) 6.0m to 7.5 m depth1$((\pi/4) \times 5.1)^2$6iv) 7.5m to 8.5 m depth1$((\pi/4) \times 5.1)^2$6iv) 7.5m to 8.5 m depth1$((\pi/4) \times 5.1)^2$7Total excal 4.60 m and Outer diameter is 5.20 m 4.60 m Ring Beam1$(\pi/4) \times 4.60^2$1RCC quantity in Ring Beam1$(\pi/4) \times 4.60^2$7Total Quantity on Total Quantity on1$(\pi/4) \times 5.1^2$</td><td>Sr. No.Item of workNos.Lengthwidth(i)Excavation in soft murum0R Area1i) up to 1.5 m depth1$((\pi/4) \times 5.20^2)$ sq. m.2ii) 1.5m to 3.0 m depth1$((\pi/4) \times 5.20^2)$ sq. m.3iii) 3.0m to 4.5 m depth1$((\pi/4) \times 5.20^2)$ sq. m.4iv) 4.5m to 6.0 m depth1$((\pi/4) \times 5.20^2)$ sq. m.5iv) 6.0m to 7.5 m depth1$((\pi/4) \times 5.20^2)$ sq. m.6iv) 7.5m to 8.5 m depth1$((\pi/4) \times 5.20^2)$ sq. m.6iv) 7.5m to 8.5 m depth1$((\pi/4) \times 5.20^2)$ sq. m.7Total excavation of second depth1$((\pi/4) \times 5.20^2)$ sq. m.1R.C.C. Ring beam (RCC M20)The Ring Beam has size of 0.3 m x 0.3m. The in 4.60 m and Outer diameter is 5.20 m.1RCC quantity in Ring Beam1$(\pi/4) \times (5.20^2 -$ 4.60²) sq. m.1RCC quantity in Ring Beam1$(\pi/4) \times (5.20^2 -$ 4.60²) sq. m.</br></br></td><td>Sr. No. Item of work Nos. Length width depth (i) Excavation in soft murum 0R Area / thk. 1 i) up to 1.5 m depth 1 $((\pi/4) \times 5.20^2)$ sq. m. 1.5 m 2 ii) 1.5m to 3.0 m depth 1 $((\pi/4) \times 5.20^2)$ sq. m. 1.5 m 3 iii) 3.0m to 4.5 m depth 1 $((\pi/4) \times 5.20^2)$ sq. m. 1.5 m 4 iv) 4.5m to 6.0 m depth 1 $((\pi/4) \times 5.20^2)$ sq. m. 1.5 m 5 iv) 6.0m to 7.5 m depth 1 $((\pi/4) \times 5.20^2)$ sq. m. 1.5 m 6 iv) 7.5m to 8.5 m depth 1 $((\pi/4) \times 5.20^2)$ sq. m. 1.5 m 6 iv) 7.5m to 8.5 m depth 1 $((\pi/4) \times 5.20^2)$ sq. m. 1.0 m Total excavation of soft rock (ii) R.C.C. Ring beam (RCC M20) The Ring Beam has size of 0.3 m x 0.3m. The inner dian 4.60 m and Outer diameter is 5.20 m. 1 $(\pi/4) \times (5.20^2 - 0.3 m.)$ 0.3 m. 1 RCC quantity in Ring Beam 1 $(\pi/4) \propto (5.20^2 - 0.3 m.)$ 0.3 m. </td><td>Sr. No. Item of work Nos. Length width OR Area depth / thk. Quantity (i) Excavation in soft murum 0 0 Area / thk. Quantity 1 i) up to 1.5 m depth 1 $((\pi/4) \times 5.20^2)$ sq. m. 1.5 m 31.86 cu. m. 2 ii) 1.5m to 3.0 m depth 1 $((\pi/4) \times 5.20^2)$ sq. m. 1.5 m 31.86 cu. m. 3 iii) 3.0m to 4.5 m depth 1 $((\pi/4) \times 5.20^2)$ sq. m. 1.5 m 31.86 cu. m. 4 iv) 4.5m to 6.0 m depth 1 $((\pi/4) \times 5.20^2)$ sq. m. 1.5 m 31.86 cu. m. 5 iv) 6.0m to 7.5 m depth 1 $((\pi/4) \times 5.20^2)$ sq. m. 1.5 m 31.86 cu. m. 6 iv) 7.5m to 8.5 m depth 1 $((\pi/4) \times 5.20^2)$ sq. m. 1.0 m 21.24 cu. m. 6 iv) 7.5m to 8.5 m depth 1 $((\pi/4) \times 5.20^2)$ sq. m. 1.0 m 21.24 cu. m. 1 RCC. Ring beam (RCC M20) The Ring Beam has size of 0.3 m x 0.3m. The inner diameter of well is 4.60 m and Outer diameter is 5.20 m. 0.3 m. 1.39 cu. m. 1</td><td>Sr. No. Item of work Nos. Length width depth Quantity (i) Excavation in soft murum 0R Area / thk. Quantity 1 i) up to 1.5 m depth 1 $((\pi/4) \times 5.20^2)$ sq. m. 1.5 m 31.86 cu. m. 2 ii) 1.5m to 3.0 m depth 1 $((\pi/4) \times 5.20^2)$ sq. m. 1.5 m 31.86 cu. m. 3 iii) 3.0m to 4.5 m depth 1 $((\pi/4) \times 5.20^2)$ sq. m. 1.5 m 31.86 cu. m. 4 iv) 4.5m to 6.0 m depth 1 $((\pi/4) \times 5.20^2)$ sq. m. 1.5 m 31.86 cu. m. 5 iv) 6.0m to 7.5 m depth 1 $((\pi/4) \times 5.20^2)$ sq. m. 1.5 m 31.86 cu. m. 6 iv) 7.5m to 8.5 m depth 1 $((\pi/4) \times 5.20^2)$ sq. m. 1.0 m 21.24 cu. m. 7 the Ring Beam has size of 0.3 m x 0.3m. The inner diameter of well is 4.60 m and Outer diameter is 5.20 m. 1.39 cu. m. 1 RCC quantity in Ring Beam 1 $(\pi/4) \times (5.20^2 - 4.60^2)$ sq. m. 0.3 m. 1.39 cu. m.</td></tdi<>	Sr. No.Item of workNos.Length OR(i)Excavation in soft murum1i) up to 1.5 m depth1 $((\pi/4) \times 5.1)^2$ 2ii) 1.5m to 3.0 m depth1 $((\pi/4) \times 5.1)^2$ 3iii) 3.0m to 4.5 m depth1 $((\pi/4) \times 5.1)^2$ 4iv) 4.5m to 6.0 m depth1 $((\pi/4) \times 5.1)^2$ 5iv) 6.0m to 7.5 m depth1 $((\pi/4) \times 5.1)^2$ 6iv) 7.5m to 8.5 m depth1 $((\pi/4) \times 5.1)^2$ 6iv) 7.5m to 8.5 m depth1 $((\pi/4) \times 5.1)^2$ 7Total excal 4.60 m and Outer diameter is 5.20 m 4.60 m Ring Beam1 $(\pi/4) \times 4.60^2$ 1RCC quantity in Ring Beam1 $(\pi/4) \times 4.60^2$ 7Total Quantity on Total Quantity on1 $(\pi/4) \times 5.1^2$	Sr. No.Item of workNos.Lengthwidth(i)Excavation in soft murum0R Area1i) up to 1.5 m depth1 $((\pi/4) \times 5.20^2)$ sq. m.2ii) 1.5m to 3.0 m depth1 $((\pi/4) \times 5.20^2)$ sq. m.3iii) 3.0m to 4.5 m depth1 $((\pi/4) \times 5.20^2)$ sq. m.4iv) 4.5m to 6.0 m depth1 $((\pi/4) \times 5.20^2)$ sq. m.5iv) 6.0m to 7.5 m depth1 $((\pi/4) \times 5.20^2)$ sq. m.6iv) 7.5m to 8.5 m depth1 $((\pi/4) \times 5.20^2)$ sq. m.6iv) 7.5m to 8.5 m depth1 $((\pi/4) \times 5.20^2)$ sq. m.7Total excavation of second depth1 $((\pi/4) \times 5.20^2)$ sq. m.1R.C.C. Ring beam (RCC M20)The Ring Beam has size of 0.3 m x 0.3m. The in 4.60 m and Outer diameter is 5.20 m.1RCC quantity in Ring Beam1 $(\pi/4) \times (5.20^2 - $ 4.60 ²) sq. m.1RCC quantity in 	Sr. No. Item of work Nos. Length width depth (i) Excavation in soft murum 0R Area / thk. 1 i) up to 1.5 m depth 1 $((\pi/4) \times 5.20^2)$ sq. m. 1.5 m 2 ii) 1.5m to 3.0 m depth 1 $((\pi/4) \times 5.20^2)$ sq. m. 1.5 m 3 iii) 3.0m to 4.5 m depth 1 $((\pi/4) \times 5.20^2)$ sq. m. 1.5 m 4 iv) 4.5m to 6.0 m depth 1 $((\pi/4) \times 5.20^2)$ sq. m. 1.5 m 5 iv) 6.0m to 7.5 m depth 1 $((\pi/4) \times 5.20^2)$ sq. m. 1.5 m 6 iv) 7.5m to 8.5 m depth 1 $((\pi/4) \times 5.20^2)$ sq. m. 1.5 m 6 iv) 7.5m to 8.5 m depth 1 $((\pi/4) \times 5.20^2)$ sq. m. 1.0 m Total excavation of soft rock (ii) R.C.C. Ring beam (RCC M20) The Ring Beam has size of 0.3 m x 0.3m. The inner dian 4.60 m and Outer diameter is 5.20 m. 1 $(\pi/4) \times (5.20^2 - 0.3 m.)$ 0.3 m. 1 RCC quantity in Ring Beam 1 $(\pi/4) \propto (5.20^2 - 0.3 m.)$ 0.3 m.	Sr. No. Item of work Nos. Length width OR Area depth / thk. Quantity (i) Excavation in soft murum 0 0 Area / thk. Quantity 1 i) up to 1.5 m depth 1 $((\pi/4) \times 5.20^2)$ sq. m. 1.5 m 31.86 cu. m. 2 ii) 1.5m to 3.0 m depth 1 $((\pi/4) \times 5.20^2)$ sq. m. 1.5 m 31.86 cu. m. 3 iii) 3.0m to 4.5 m depth 1 $((\pi/4) \times 5.20^2)$ sq. m. 1.5 m 31.86 cu. m. 4 iv) 4.5m to 6.0 m depth 1 $((\pi/4) \times 5.20^2)$ sq. m. 1.5 m 31.86 cu. m. 5 iv) 6.0m to 7.5 m depth 1 $((\pi/4) \times 5.20^2)$ sq. m. 1.5 m 31.86 cu. m. 6 iv) 7.5m to 8.5 m depth 1 $((\pi/4) \times 5.20^2)$ sq. m. 1.0 m 21.24 cu. m. 6 iv) 7.5m to 8.5 m depth 1 $((\pi/4) \times 5.20^2)$ sq. m. 1.0 m 21.24 cu. m. 1 RCC. Ring beam (RCC M20) The Ring Beam has size of 0.3 m x 0.3m. The inner diameter of well is 4.60 m and Outer diameter is 5.20 m. 0.3 m. 1.39 cu. m. 1	Sr. No. Item of work Nos. Length width depth Quantity (i) Excavation in soft murum 0R Area / thk. Quantity 1 i) up to 1.5 m depth 1 $((\pi/4) \times 5.20^2)$ sq. m. 1.5 m 31.86 cu. m. 2 ii) 1.5m to 3.0 m depth 1 $((\pi/4) \times 5.20^2)$ sq. m. 1.5 m 31.86 cu. m. 3 iii) 3.0m to 4.5 m depth 1 $((\pi/4) \times 5.20^2)$ sq. m. 1.5 m 31.86 cu. m. 4 iv) 4.5m to 6.0 m depth 1 $((\pi/4) \times 5.20^2)$ sq. m. 1.5 m 31.86 cu. m. 5 iv) 6.0m to 7.5 m depth 1 $((\pi/4) \times 5.20^2)$ sq. m. 1.5 m 31.86 cu. m. 6 iv) 7.5m to 8.5 m depth 1 $((\pi/4) \times 5.20^2)$ sq. m. 1.0 m 21.24 cu. m. 7 the Ring Beam has size of 0.3 m x 0.3m. The inner diameter of well is 4.60 m and Outer diameter is 5.20 m. 1.39 cu. m. 1 RCC quantity in Ring Beam 1 $(\pi/4) \times (5.20^2 - 4.60^2)$ sq. m. 0.3 m. 1.39 cu. m.						